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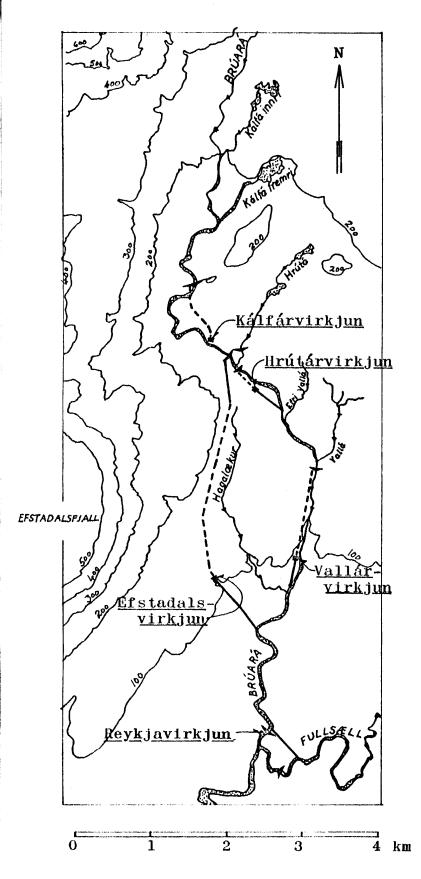
APPRAISAL REPORT ON THE EFSTIDALUR HYDRO-ELECTRIC PROJECT UPPER BRUARA RIVER. ICELAND

English version of an excerpt from an Appraisal Report to the State Electricity Authority, on the hydro-electric potential of the upper reaches of Brúará River dated May 1962.

> English version prepared by the State Electricity Authority Sept. 1962

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Skýringar LEGEND

- → Stifla DAM
- Orkuver POWERPLANT
- Skurður CANAL
- --- Pipa PIPE

THE STATE ELECTRICITY AUTHORITY
BRUARA DEVELOPMENT
PLAN OF UPPER BRUARA

RAFORKUMÁLASTJÓRI

VIRKJUN BRØARÅR YFIRLITSMYND AF EFRI BRØARÅ

T.: 5. F. R.: Y.: S.: 4. Dags. 13.2. '62

SIGURDUR THOROEDSEN . VERKFRÆÐISTOFA Miklabraut 34 . Reykjavík

Nr. A 2297

Summary of Mydro-electric Developments in the Upper Reaches of Briand River, showing Capacity, Energy Output and Costs

											The second secon
Scheme	e Power Plants	Head-water elev.	Tail- water elev.	Gross head m	Installed Energy capacity output MW GWh/a		Cost of product. plant Mar.	Cost of Total transm. const plant cost Mcr. Mcr.	Total construct. cost	Total Energy construct.cost from cost generator Mcr. aur/kWh	Energy cost at Sog aur/kWn
≪	Kalfa Project Efstidalur Project Reykir Project, 1. alt.	182 156 80.5	385	160 22 80.5 75.5 73 7.5	3.80 2.56 5.56	24 146 20	\$283 \$284		251	だれる	18
	Total, Scheme A	•	•	105	23.76	190	2 4 5	42	291	18	22
M	Kalfa Project (-Scheme A) Hruta Project, 1.alt. Walla Project Reykir Project, 2.alt	85 55 55 87 55 55 87 55	3725	8888 8	బగ్గాల్లు కోన్ కోట్	42°C	පැස්සී			చ్రి చెద్ద	• • • •
	Total, Scheme B	•	•	91.2	21.83	175	741	43	384 384	19	24
Ö	Kalfa Project (*Scheme A) Hruta Project, 2.alt. 160 Walla Project (*Sch.B)120 Reykir Project, 2.alt	182 160 3) 120	127 127 84	arx s	67.79 9.90 83.88	485	72 102 83 83	: R	393	32 166 166 167	: &
	Total, Scheme C			1 1	24.18	194	267	4	411	19	23

Notes:

1. Annual charges assumed 10% of first cost 2. Transmission losses from plant to Sog assumed 7% 5. Import duties & taxes are included in first cost

Rate of Exchange: 1 U.S. \$ = 45 Icel. kronur; 1 U.S. mill = 4.5 Icel. aurar (aur)

The abbreviation "Mkr." is used throughout this Report for millions of Icelandic kronur"

Table of Significant Data of Efstidalur Project

Normal headwater elevation "tailwater" Discharge at rated capacity Normal gross head "net head		157 m 80.5 m 30 kl/s 76.5 m 71 m
Type Length of earth dam " " spillway Elevation of spillway crest " earth dam crest Maximum dam height Spillway capacity (h= 1.3 m)		oncrete spillway 400 m 60 m 157 m 159 m 7 m 195 kl/s
Water conductors Intake canal Length Width of bottom Slopes Maximum depth		550 m 11 m 1:3 3 m
Intake Elevation of threshold secondary Intake opening Speed in intake opening Gate type	otion wheeled gate	4.6x5 m ² 1.3 m/s 16 m ²
Headrace pipe Type Length Cross section area Diameter Flow velocity	wood stave pipe,	steel girdled ab. 2450 m ₂ 11.4 m ² 3.8 m 2.65 m/s
Surge tower Type Diameter Cross section area Bottom elevation Water level, normal """ maximum """ minimum	steel tank, concr	15 m ₂ 177 m 140.2 m 154.0 m 168.3 m 147.0 m
Penstock Type Diameter at upper end " " lower end Length		steel pipe 3.3 m 2.9 m 150 m
Tailrace canal Type Length Width at bottom Maximum depth	excavated in soil 5 m and	& rock; unlined ab.1040 m 20 m 25 m

Head los	ses					
	head	losses	at 100% flow		5.5	m
11	11	11	in % of gros	s head	7.2%	
Ħ	H	11	at 90% flow		4.5 1 5.8%	m
H	Ħ	11	in % of gros	s head	5.8%	
Power hou	ıse					
Type Turbine	,			Semi	-underground	
Type Net he		ordinary			with vertice 71.5	m
Power Speed	at or	rd. net	head and 30	kl/s	25 800 H 250 R	.P. (m) PM
Generato	or					
Capac:	lty				22.7	AVM
P.F.					0.8	
Voltag	ze				10.5	
Speed					250	RPM
Main tra	nsfor	mer				
No. of	wind	lings			3	
		? phase:	3		3 3 22.7/2/22.7 10.5	
Capac			. .	;	22.7/2/22.7	MVA
Voltag		V. wind	ling		10.5	kV
'n	; I.	· Y •			11	
**	; H.	.v. "			66	kΨ

Sigurõur Thoroddsen Engineering Constultant, Member FIDIC Miklabraut 34 Reykjavík - Iceland.

Reykjavík, May 27 1962

Summary Letter.

The State Electricity Authority Laugavegur 118 Reykjavík

Gentlemen.

We are forwarding with this letter an Appraisal Report on the hydroelectric development of the upper reaches of Bruara River, above its confluence with Hagaos River, the outlet of Lake Apavatn. However, we wish to emphasize that the appraisals of the Kalfarvirkjun and Reykjavirkjun plants are of a more preliminary nature than those of the other plants, and that these two projects were included merely for the sake of completeness.

For the Dynjandi Project farther downstream on the Bruard, we have previously sent you a similar report.

Three schemes have been drawn up for the over-all development of the reach of Bruara River in question, each consisting of 3-4 seperate developments. In a summary table at the front of this Report, the main data of each of these schemes are shown.

According to this summary, the total installed aspacity of all power plants envisaged is 21.83 - 24.18 MW, depending on the over-all scheme adopted, the total annual energy output in a dry year 175 - 194 GWh; the first Cost of production plant alone Mkr. 341- 367 but Mkr. 384-411, also by schemes, when the transmission plant

required to transmit the power to Irafoss on the Sog is included. Assuming annual charges amounting to 10% of first cost, the energy cost from generators would be 18 - 19 aurar/kWh, but 22 - 24 aurar/kWh at the 138 kV busbar at Irafoss, again depending on the scheme. This low energy cost for hydro-plants of the size in question here, is caused by the favourable conditions for development found on the Bruará River.

Of the individual projects, the most promising ones appear to be the Efstidalur Project in Scheme A, and the Hruta Project, 2. alternative plus the Valla Project in Scheme C. These two schemes are mutually exclusive, as far as these Projects are concerned.

The Efstidalur Project has a rated capacity of 18.2 MW and an energy output in a dry year of 146 GWh. Estimated construction cost of production plant is Mkr. 222, that of production and transmission plant together Mkr. 251. Estimated energy cost, assuming annual charges to be 10% of first cost is 15 aurar/kWh from generator and 18 aurar/kWh at Irafoss.

Similar figures for the two-stage alternative, Hrútá plus Vallá, are: Installed capacity of both plants 17.6 MW; annual energy 141 GWh; first cost of production plant Mkr. 230; of production plus transmission plant Mkr. 262. Energy cost from generator 16 aurar/kWh; at Irafoss 20 aurar/kWh.

The cost of transmitting the power from Sog to Reykjavík is not included in the above figures. The reason is that it may be assumed that a second transmission line from Sog to Reykjavík will be constructed in a few years, for the reason of reliability of supply in the Reykjavík area, regardless of whether or not the Projects in question here will be constructed. Only the incremental cost of transmitting the extra power over the two lines would be

chargeable to the Projects, and this incremental cost will presumably be very low, since each line would operate at little more than half its thermal capacity.

All import duties and taxes on materials, plant and equipment brought in from abroad for the Projects are included in the cost figures.

Stream-flow records for Bruara River at the sites of the Projects here discussed are not available. A water stage recorder was installed near the Efstidalur site in Aug. 1961. When this recorder has been in operation for a suitabel time, a correlation between the discharge there and at Dynjandi may be established. At the latter place records are available for 14 years, and they may then be "adjusted" with the aid of this correlation. The capacity and energy figures stated above are based on assumptions about the stream-flow. It is felt that these assumptions are conservative rather than the opposite, and this seems to be confirmed by the records of the Efstidalur recorder since its installation, although too much significance should not be attached to records of so short a duration.

Bruara River is almost a pure linda river in the reach in question here, being fed to nearly 100% by springs issuing from the ground a short distance above the intakes. There will thus be neither troubles from ice nor sedimentation in the operation of the plants.

Our studies strongly indicate that the Projects in question here are very favourable, especially the Efstidalur Project or the Hruta plus Valla Projects. All the sites are located in inhabitated regions, as short distance from existing motor roads. The donditions for the actual construction work are thus favourable. Mapping, subsurface explorations and other investigations are well advanced and may easily

be completed in short time. Since all the Projects are of the surface type, with all major structures at ground surface, the risks due to unsound soil or rock formations are less here than at various sites that have been under consideration in the past few years.

Each of the Projects could easily be completed in the course of 1 - 2 years.

A sel

With this in mind, we are of the opinion that at least some of these Projects in the upper part of Bruara River are sufficiently favourable as to merit further study for the purpose of ascertaining whether or not some of them might be suitable to meet the power requirements of Southern Iceland in the immediate future.

Sincerely yours,

Sigurður Thoroddsen (sign)

1. Introduction. Location of Site.

The Efstidalur Project is situated in the upper reaches of Bruará River, a tributary to the Hvítá River, Southern Iceland. The geographic location is shown on the location map, drwg. No. A 2297, at the beginning of this Report. Of the other Projects shown on the map, Kálfárvirkjun and Reykjavirkjun are developments of the reaches of Bruará River immediately upstream and downstream, respectively, of the Efstidalur Project, while the Hrutárvirkjun and the Vallárvirkjun together represent a two-stage alternative to the Efstidalur Project. This alternative was found to possess no advantages over a single-stage development. These other Bruará Projects will not be discussed in the present Report.

2. Stream Flow. Power and Energy.

Bruara River is a typical linda river, i.e. it is fed almost entirely by springs. Surface run-off is confined to the snowmelting period. Accordingly, the flow is extremely uniform.

Due to pervious geologic formations covering parts of the water divides their exact location and the size of the drainage area cannot be accurately determined.

Prior to initiation of our design, the following results of individual streamflow measurements were made available to us by the State Electricity Authority's Hydrologic Survey (S.E.A.H.S.).

Date of measurements	Feb.16 1958	Aug. 11. 1960	Sept. 21. 1961
Stream-flow in kilolitres per second (kl/s)			
Brúará River, above conflu- ence with Hrútá River	18,5	approx. 22	23,5
Hrútá River l km approx. above confluence with Brúará River	7,3	8,3	8,3
Hrútá River, at confluence with Brúará River			9,0
Brúará River, at highway bridge near tailrace of			
proposed plant			37,3
Brúará River at Dynjandi			5 3, 8

The greatest floods occurring in Bruara River in the development area considered here were estimated by S.E.A.H.S. at 200 kl/s approximately.

The uniform flow of Bruara River; the relatively long water conductors and the small pondage available all indicate that the plant should be designed for a high plant factor, i.e. base load operation. We selected a design plant factor of 91% (8000 hours), and a flow of 30 kl/s at rated plant capacity. This gives a rated generator capacity of 18.2 MW and an annual energy output of 146 GWh.

The figures were selected on the basis of a dry year. In a normal year, the plant factor would approach 100%.

Since only a very limited pondage is available at the plant, firm capacity is somewhat less than 18,2 MW.

When our work on the Report was well advanced, the first stream flow records of a water level recorder installed near the site in Aug. 1961 became available. Although of a short duration, owing to the extreme uniformity of the flow, these records are valuable in establishing a correlation between the flow at the site and that at Dynjandi, where the stream-flow has been measured for 14 years.

A preliminary comparison seems to indicate that the capacity and energy output figures are conservative and may require modification in a final design. However, because of the very short duration of simultaneous records, an uprating of the Project was not felt justified at this appraisal stage.

The water temperature at the intake will be above zero centigrade throughout the year, owing to the spring water feeding the river immediately upstream.

For similiar reasons there is no sediment transport. Operating troubles caused by ice or sedimentation will therefore be non-existent.

3. Description of Efstidalur Project

3.1 Dam and Intake Canal (Drwgs A-2279; 2280)

The damsite was selected some 200 m above the confluence of the Bruara and Hruta Rivers. The dam extends across both rivers and the tonguenbetween them.

Information about the geology at the damsite as well as at the sites of other structures of the Efstidalur Project, was obtained from Mr. Haukur Tomasson, geologist of the S.E.A.

Both rivers, Bruara and Hruta, are flowing on basalt rock, but the left bank of Hruta River is of moraine, upon which the left end of the dam will abut.

The dam is an earth-fill dam, except for a concrete spillway formed as a gravity dam across the Bruará river-bed. The length of the spillway is 122 m; its crest is at el. 157 m. The crest of the earth-fill sections at el. 159 m.

The greatest flood occurring at the damsite is not known, but at the Dynjandi gauging station, farther downstream on the Bruara, the maximum observed flood during a 10-year observation period was 194 kl/s. In view of that, the spillway, as described above, should have ample discharging capacity.

A cut-off trench beneath the dam, down to bedrock, is contemplated. In the bottom of the trench, the rock will be tightened with cement sludge. The impervious core of the dam will be of lossy soil, with sand and gravel filters on both sides. The bulk of the fill will consist of compacted moraine and gravel, which materials are found in ample quantities near the damsite. However, no sampling and testing of these local materials have as yet been carried out.

The intake canal, which leads southward from the pond, is excavated to a bottom elevation of 154 m. Its left bank is formed as an earth-fill embankment with filters and drains, and is in a direct continuity of the dams across the rivers. The right bank is retained by the slopes of Efstadalsfjall.

The crest of the left bank is at el. 160 m.

According to results of diamond core drilling and permeability tests, the basalt which forms the bedrock at the sites of the dam and the intake canal, is very cracked and pervious down to a depth of 30-40 m, offering very little resistance to the ground-water flow. Where moraine overlies the bedrock, the water is under artesian pressure. It has been assumed that rock grouting can essentially be limited to the end of the intake canal, since the structures in question here are comparatively low; the height of the dam being less than 5 m at most places.

3.2 Intake (Drwg A-2281)

The intake is situated in the middle of a retaining wall lying athwart to the canal alignment, at its lower end.

The retaining wall, which is of concrete, extends down to bedrock. Beneath the wall, the rock is tightened by cement grouting. The canal is made deeper near the end; creating a suitable depth at the intake.

The intake is of a traditional and simple design, since no operating troubles can arise, from ice or other causes. The flow velocity through the thrashracks will be ab. 1.5 m/s.

The intake gate will be of the quick-closing type, hydraulically operated and remotely controlled. An air-inlet is located below the gate. The inlet may be used as a manhole.

A shelter, 7 m by 5 will house the hoisting equipment.

3.3 Headrace Pipe (Drwgs A-2279; 2284)

The headrace pipe will be a wood-stave pipe, steel girdled, 3.3 m dia. Its length from intake to surge tower 1s ab. 2450 m. It will follow the landscape; for the most of its length it lies on a gravel terrace along Efstadalsfjall. Then it lies across a depression to the surge tower, which is sited on a hillock some distance to the north of farm Efstidalur (see drwg.)

On its route, the pipe crosses three brook channels, dry except during snow-melt. The largest of them has to be bridged, whereas culverts will suffice in the other two.

Since the pipe will curve up and down with the landscape along its route, it must at two places be fitted with draining valves and at two places with air outlet.

The pipe will be bedded up to its middle in a drained gravel fill. A grass-covered earth-fill, 50 cm thick on top, with side slopes of 1:1.5, will cover the pipe.

The pipe will be connected to the intake at one end and to the surge tower at the other, by steel tubes. Manholes will be on the pipe at every 300 m.

3.4 The Surge Tower (Drwg A-2281)

The surge tower is a circular steel tank, 15 m dia, on a concrete foundation. The height of the tank is 28 m; of the foundation 6 m. The duct through the foundation will be steel lined, 3.8 m dia at the pipe end tapering down to 3.3 m downstream of the tank, where a quick-closing valve will be inserted. The valve is housed in a shelter, 7.5 m by 10.5 m. Below the valve, the pipe is fitted with an air-duct, which lies into the tank and up above its top, to protect the duct against frost.

3.5 Penstock

Below the surge tank and the quick-closing valve is the penstock, a steel pipe 150 m in length, 3.3 m dia. The penstock is straight in plan but with two curves in elevation: One at the turbine; the other at an edge of a flat slope some distance above the powerhouse, where the penstock will be fastened by a concrete anchor. Between the anchor and the valve, the pipe rests on ring girders supported by steel columns, pivoted at their lower ends and founded on concrete blocks. On this part, the pipe will be equipped with one expansion joint. Thermal Insulation of this part of the pipe is not contemplated, since it is not required; there being no danger that the water will freeze. From the anchor to the turbine, the penstock will be embedded in concrete.

3.6 The Powerhouse. (Drwgs A-2282; 2283)

The powerhouse is situated in a depression or a gullet which cuts into the hills along Efstadalsfjall.

The gullet is deepened by excavation and connected to Bruará River by a deep canal that will become the tailrace canal of the plant. The powerhouse is at the upper end of this canal, where it has reached a depth of 25-30 m.

The powerhouse is conventional in all respects and does not call for a detailed description; reference is made to the drawings.

The machine halls lies athwart to the canal. A side building is on its eastern side.

Besides the machine hall with an erection bay the powerhouse also contains a control room, a switchgear room, offices, hall and a washroom on the upper floor. A side room houses the step-up transformer. From that room, the transformer may be moved into the erection bay for maintenance and overhaul. For removing the core, the transformer may be lifted into a pit in the floor of the erection bay.

On the lower floor of the side building there is a work-shop, store rooms and a room for the station supply transformers. Below this floor is also a small basement, which may be used as a store room, and to give access to the bulkhead gate.

3.7 The Tailrace Canal (Drwgs A-2279; 2284)

The tailrace canal leads the water back to Bruara River. Its length is little over 1000 m. The lowest part of the canal crosses the river twice, and for the last 250 m the canal lies practically in the riverbed, being merely a deepening of the natural channel. At this place, the river bottom consists of "deigulmór", which expectedly is easily excavable with suitable equipment.

As already stated, the canal is very deep, some 30 m by the powerhouse, but the depth decreased as the surrounding ground gets lower.

The soil on the canal route has been partly explored, as indicated on the drawings.

The bottom width of the canal is 5 m. Side slopes of 4:1 in rock and 1:2 in soil have been assumed. Besides, the rock is stripped of soil for a distance of 5 m from the rock excavation. Of course, the canal must be fenced off and other precautions taken to prevent accidents.

3.8 Roads

An access road to the powerhouse will lie along the right bank of the tailrace canal. Its length from the highway to the powerhouse will not exceed 1000 m. From the powerhouse, the road will follow the pipeline to the intake works and the dam. The length of this latter part will be some 3500 m. No noteworthy difficulties should, therefore, arise in connection with the transport of materials and equipment to the site.

3.9 Mechanical and Electrical Plant and Equipment

3.9.1 Turbine

The power plant is to contain one aggregate. The chief characteristics of the turbine are as follows:

Type - Francis
Arrangement - vertical shaft
Net head - 71.5 m
Rated capacity - 25 800 HP metric
Efficiency at rated capacity - 0,90
Turbine flow at rated capac.
and net head of 71.5 - 30 kl/s
Speed - 250 RPm

3.9.2 Generator

Rated capacity -22.7 MVA Efficiency at rated capacity -0,96 P.F. -0,8 No. of phases -3 Frequency -50 Hz Voltage -10,5 kV Speed -250 RPm Type -Vertical shaft 17.

3.9.3 Connection of Electrical Equipment

Plan A-2292 illustrates the single line diagram of the power plant. The generator will be connected to a 10.5 kV single busbar from which cables will lead to the main transformer. The 10.5 kV switchgear is of the indoor type, in sheet steel cubicles.

3.9.4 Main Transformer. Switchgear

The main transformer of the power plant is a three-winding transformer, rated 10,5/11/66 kV, 22.7/2/22.7 MVA.

The 11 kV winding is contemplated for a rural line from the plant. If, however, it should not be necessary to make an arrangement for this possibility, an ordinary two winding transformer could be used, leaving out the 11 kV winding.

The transformers are placed in a room off the erection bay in the machine hall re drwg. A-2283.

The switchward with circuit breakers, switches current and potential transformers and other equipment for outgoing 66 and 11 kV lines is placed near the power house.

3.9.5 Station Supply

The station supply comes from the 10,5 kV bus-bars, through two 250 kVA transformers, 10,5/0,4 kV. These transformers are equipped with automatic on load voltage regulation.

The low voltage terminals are connected to 400 V bus-bars, from which the station supply is taken. These bus-bars together with circuit-breakers.and other equipment are kept in closed steel cubicles. The transformers are two for safety's sake.

All Station supply equipment is placed in a side wing of the powerhouse, see drawing.

3.9.6 Control Room

The control room of the power plant is in the previously mentioned wing, where all the necessary control equipment will be placed together with meters, relays and terminal equipment of a carrier communication over the transmission line.

3.9.7 Transmission Line and Tie-in at Irafoss

It is intended to transmit the energy to Irafoss by a single circuit 66 kV transmission line on double wood poles. The line would be about 29.2 km in length.

At Irafoss the voltage is raised to 138 kV in a special transformer, 22,7 MVA, 66/138 kV, and the line connected to the 138 kV busbars at Irafoss. This transformer is equipped with automatic on load voltage regulation.

It is to be expected that a new 138 kV line has been built from Sog to Reykjavík when the power plant begins to operate.

4. Cost

4.1 General

The cost estimates presented in this Report are in the form usually adopted by our firm to such estimates.

Included in the cost figures are all import duties and taxes on material and equipment, but excluded are riparian rights; compensations for land use and quarrying rights, and the cost of preparatory studies and site investigations.

The cost of mechanical and electrical plant and equipment was estimated on the basis of information obtained from West-German manufacturers about similar items for the Tungufell Project on the Hvítá, the Vallá Project on the Brúará (cf. page of this Report), the Dynjandi Project on the Brúará and the Árbæjarfoss Project on the Rangá. The cost of the woodstave pipeline was based on information from a Scandinavian manufacturer, Skandinavisk Trærör A/S, Sweden; that of the steel penstock and the surge tank on information received from mechanical workshops in Reykjavík that erect similar structures.

In accordance with our usual practice we have added to the direct construction cost, such items as contingences, engineering cost and interests during construction. These items were estimated as follows:

- 1. Contingencies and commissions 20% of direct cost.
- 2. Engineering, 8% + of direct cost plus contingencies.
- 3. Interest during construction, 9,5% + of direct cost plus contingencies plus engineering.

The first of these items was selected rather on the high side since the site investigations are not yet completed. The third item is also probably somewhat too high, since the construction period of the plant should not exceed 2 years.

4.2 Cost Summary

Production Plant:

Power plant structures	Mkr.	12.07
Reservoir, dams and water conductors	11	106.97
Turbine and generator	Ħ	29.40
Accessory electrical equipment	*1	3.70
Misc. power plant equipment	14	3.20
Roads	***	1.17

Total, Production Plant Mkr. 156.51

Mkr. 251.00

Transmission Plant: Efstidalur substation Mkr. 5.40 Transmission line. Efstidalur-Irafoss (Wood-pole line, 66 kV; 29.2 km) Mkr. 8.80 Extension of Irafoss substation 6.30 Total, Transmission Plant Mkr. 20.50 Total, Direct Cost Mkr. 177.01 Contingencies & Comissions ab. 20% 35.40 Mkr. 212.41 Engineering, ab. 8% 17.00 Mkr. 229.41 Interest during construction ab. 9.5% 21.59 Total Construction Cost

5. Site Investigations:

5.1 Topographic Mapping

Aerial topographic maps of the development area to a scale of 1:5000 with 2 m contours are available. A longitudinal profile of the reach of river to be developed is also available.

For final design, mapping to a larger scale of the sites of structures and their immediate vicinity will be required.

5.2 Geology

For the present design, information on the general geology of the area was obtained from S.E.A. geologist Mr. Haukur Tomasson.

Diamond core drilling near the damsite performed in the summer of 1961 showed that an underground power house near the dam and a tailrace tunnel would be a somewhat questionable arrangement. Accordingly, in the present report, a scheme has been selected, with a wood-stave

pipeline, surge tank and steel penstock leading to a surface powerhourse at the end of a tailrace canal.

Besides the core drilling, the soil layers at the site of the principal structures have been explored. This work was not completed when our report was written, but we have been kept informed about the results.

Before a final design is made, these explorations of the sites of structures will have to be completed. The job of carrying out these investigations is a rather small one.

5.3 Search for Construction Materials

Some search for construction materials in the general vicinity of Bruara River was carried out in connection with the Dynjandi Project. From this it may be concluded that construction materials in sufficient quantities and of adequate quality may be obtained within a reasonable hauling distance from the sites in question here. However, before an invitation of tender is prepared, further search and testing will be required.

5.4 Hydrology

As previously mentioned, a water stage recorder has been installed near the site of the Project. A preliminary rating curve has been prepared by S.E.A. H.S. but some more stream-flow measurements have to be made to complete the curve. An attempt should be made as soon as practicable to establish a correlation with the gauge at Dynjandi where 14 years of records are at hand.

APPENDIX

Detailed	Cost	Estimates	3

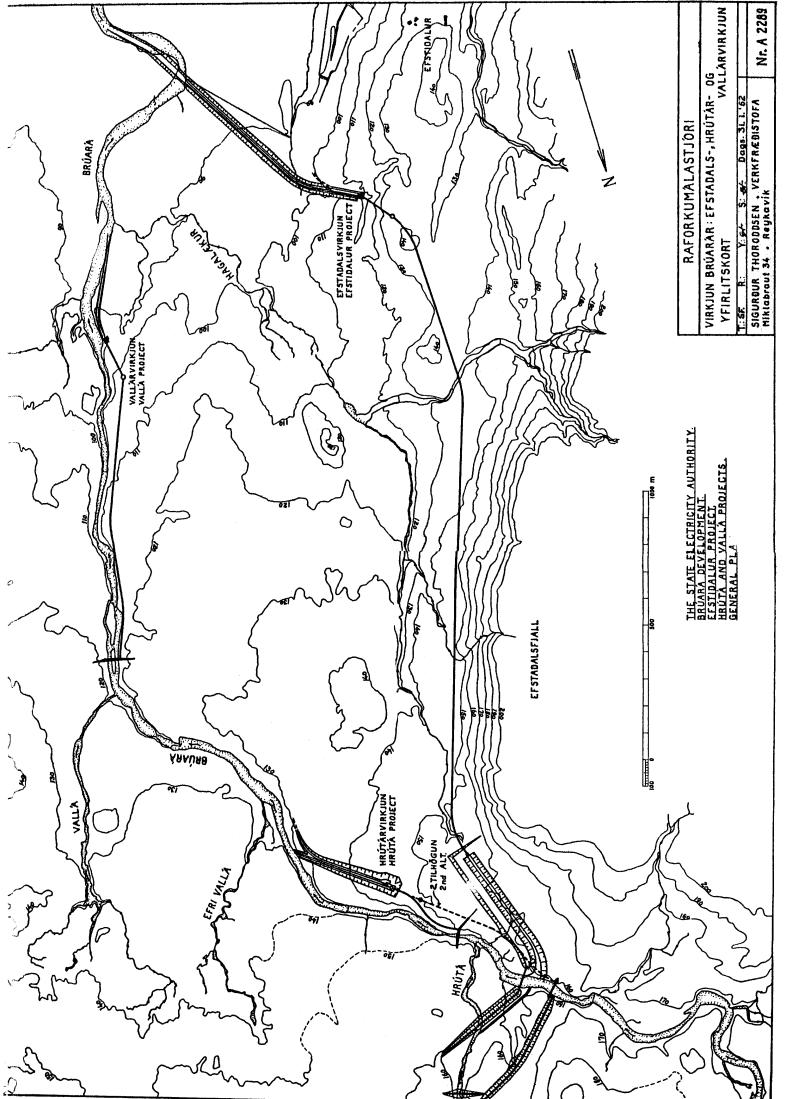
Devalled Cost Estimates					
1. Civil Engineering Works	Unit	Quantity	Unit Price Krónur	Price Krónur	Total Krónur
Dam					
Concrete dam					
Soil excavation Rock " Concrete Concrete forms, plain " " curved Steel reinforcement Concrete joint sealings Painting and finish	m3 1,3. m2 m2 m2 kg m 1.s.	4.000 2.360 1.400 440 15.000	60 1.100 270 430 13,20 1.000	378.000, 189.200, 198.000, 100.000,	- - -
Earth dam east of Bruara					
Soil excavation Rock and/or soil	_m 3	16.200	30	486.000,	•
excavation for cut-off Gravel filters and fill Earth fill Riprap lining Slush grout Finish	m3 m3 m3 m3 1.s.	1.000 9.000 5.500 4.000	300 80 80 120	300.000, 728.000, 440.000, 480.000, 50.000,	- -
Headrace canal					
Excavation in peat " gravel and	m ³	108.000	25	2700.000,	-
sand " " rock Slush grout	m3 m3	21.000 5.000	40 400	840.000,	-
Loess fill	ı,s.	10.000	100	180.000,	-
Gravel and sand filters and riprap Finish, draining and	_m 3	13.200	.80	1056.000,	-
sodding	1.8.			250.000,	_ 8106.000,-
Intake					
Preparation of rock Concrete "forms	1 _m 3 _{m2}	1.530 1.950	1100 280	60.000, 1683.000, 546.000,	• •
Steel reinforcement Grouting of rock Finish, painting, electric	kg l.s.	25.000	13,20	330.000,- 500.000,-	
installation; lights Thrashracks drains, slots airducts, pipe connector	l.s.			430.000,-	
and bulkhead gate Wheeled gate 4 m by 4 m	1.s. 1.s.			325.000,- 750.000,-	4624.000,-

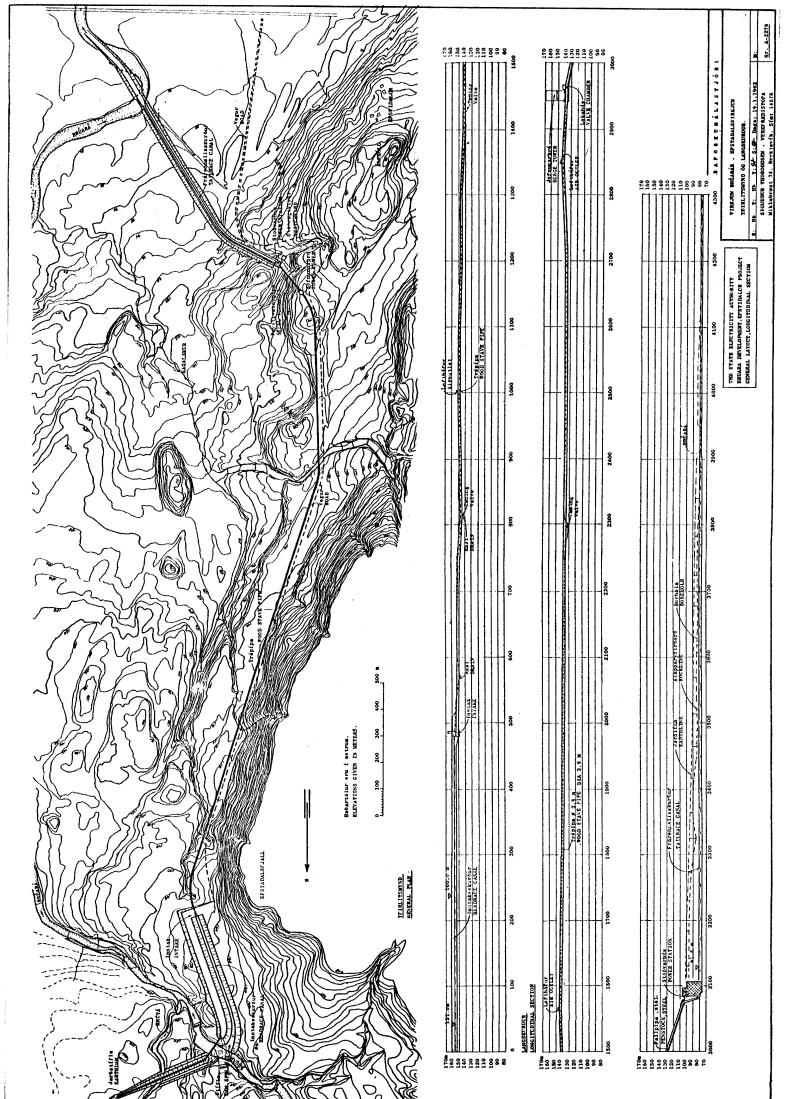
	Unii	t Quantity	Unit Price Kronur	Price Krónur	Total Krónur
Cofferdams					
Dams and pumping	1.8	•		700.000,-	700.000,-
Headrace Pipe	_				
Soil excavation Gravel fill Earth fill Drain pipes, 6" dia Laying of drain pipes Brook culverts Wood stave pipe, 3.8 m dia, with accessories,	m3 m3 m 1.s.		40 80 40 35	980.000,- 1960.000,- 1180.000,- 115.500,- 220.000,-	
delivered to site Assembling of wood-stave	m	2.450	18000	44100.000,-	
pipe Manholes Fenches and sodding	m pcs. l.s.		1310 -	3209.500,- 70.000,- 250.000,-	52.285.000,-
Surge Tower	***				
Soil excavation Concrete Concrete forms, plain " curved Steel reinforcement Steel tank, incl. erecti Steel lining Quick-closing gate;		600 1.100 400 600 11.000 21.000 20.000	30 1100 270 430 13,20 25 25	18.000,- 1210.000,- 108.000,- 258.000,- 145.200,- 3025.000,- 500.000,-	
<pre>delivered to site Mounting of gate Painting; insulation of valve chamber; doors;</pre>	1.8.			260.000,-	
windows, etc.	l.s.		*****	450.000,-	7.574.200,-
Penstock					
Excavation in soil Concrete forms Steel reinforcement Steel pipe, mounted Finish	m3 m2 kg kg 1	2.000 810 350 10.000 28.000	30 1100 270 13,20 25	60.000,- 891.000,- 94.500,- 132.000,- 3200.000,-	4.477.500,-
Power house	77				
Excavation in soil " rock Concrete " forms for turbine	カラ mラ mラ mラ mラ	4.500 10.200 600 1.000	30 300 1100 1200	135.000,- 3060.000,- 660.000,- 1200.000,-	
and draft tube	m ²	500	400	200.000,-	

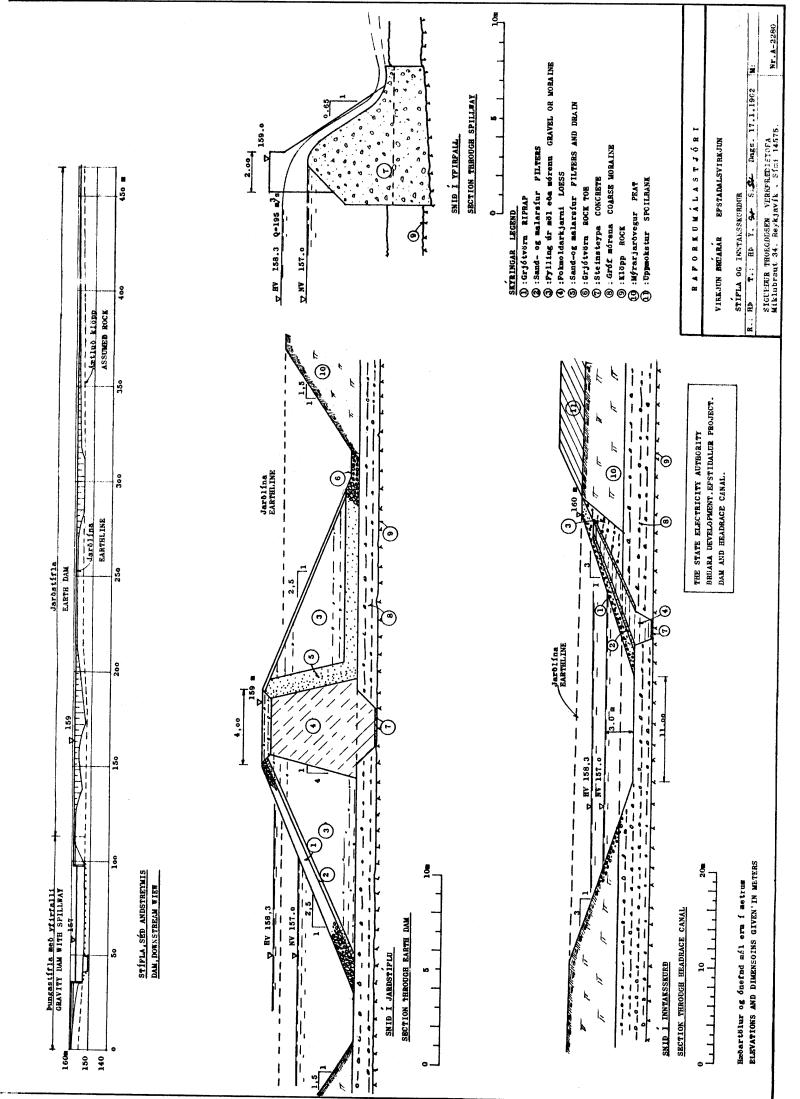
	Un	1t	Quantity	y P	nit rice ronur	Price Krónur	Total Krónur
Power house cont							
Concrete forms, other Steel reinforcement Insulation of outer			.500 .000	2 13,	70 20	1215.000,- 1848.000,-	
walls and finish Roofs, inside insulation	_m 2		400	2	00	80.000,-	
and finish " , bitumen filt Roofgutters and down-	m2		420 420		00	168.000,- 42.000,-	
pipes Painting, indoors and	l.s	•				40.000,-	
outdoors Partition walls with	1.s	•				220.000,-	
glass Flooring Floor tiles	12s m2		650 300		70 60	70.000,- 45.500,- 138.000,-	
Windows Doors - outdoors " - indoors Spiral staircase, steel staircase and various	1.s 1.s 1.s	•				180.000,- 85.000,- 25.000,-	
handrails Sanitary installations,	l.s	•				50.000,-	
water supply, etc.	l.s	•				60.000,-	9.521.500,-
Tailrace canal	7						
Excavation in soil " " rock Cofferdams, pumping	m3 m3	6	0.000 3.000		00 12	5100.000,- 2600.000,- 1000.000,-	18.700.000,-
Yard, outdoor finish, switchyard, roads							
Foundations of switchyard structures, fences, etc. Outdoor finish	. 1.					200.000,-	
Roads	1.5					600.000,- 125.000,-	1.925.000,-
Houses for station operators	1.8	3.				500.000,-	1.500.000,-
Contractor site facilities	1.5	3.				1500.000,-	4.500.000,-
Civil engineering works, total direct cost						1	20.398.400,

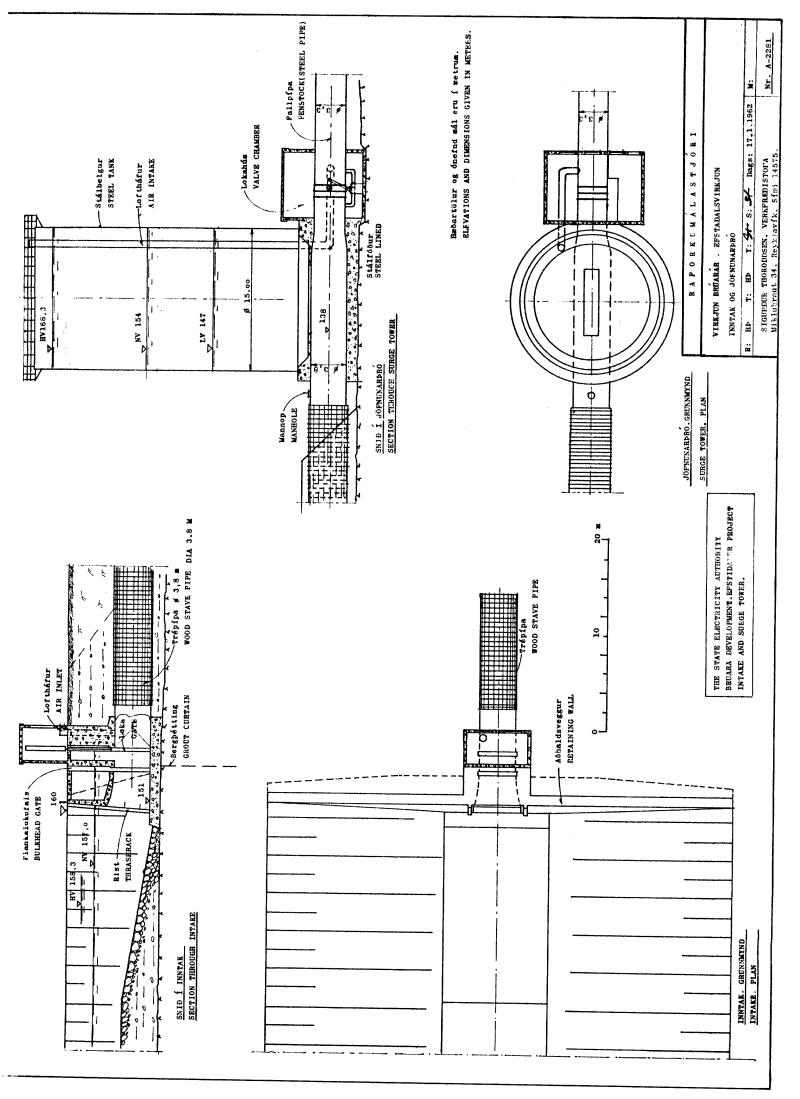
2. Machinery and electrical plant and equipment: Turbine with accessories

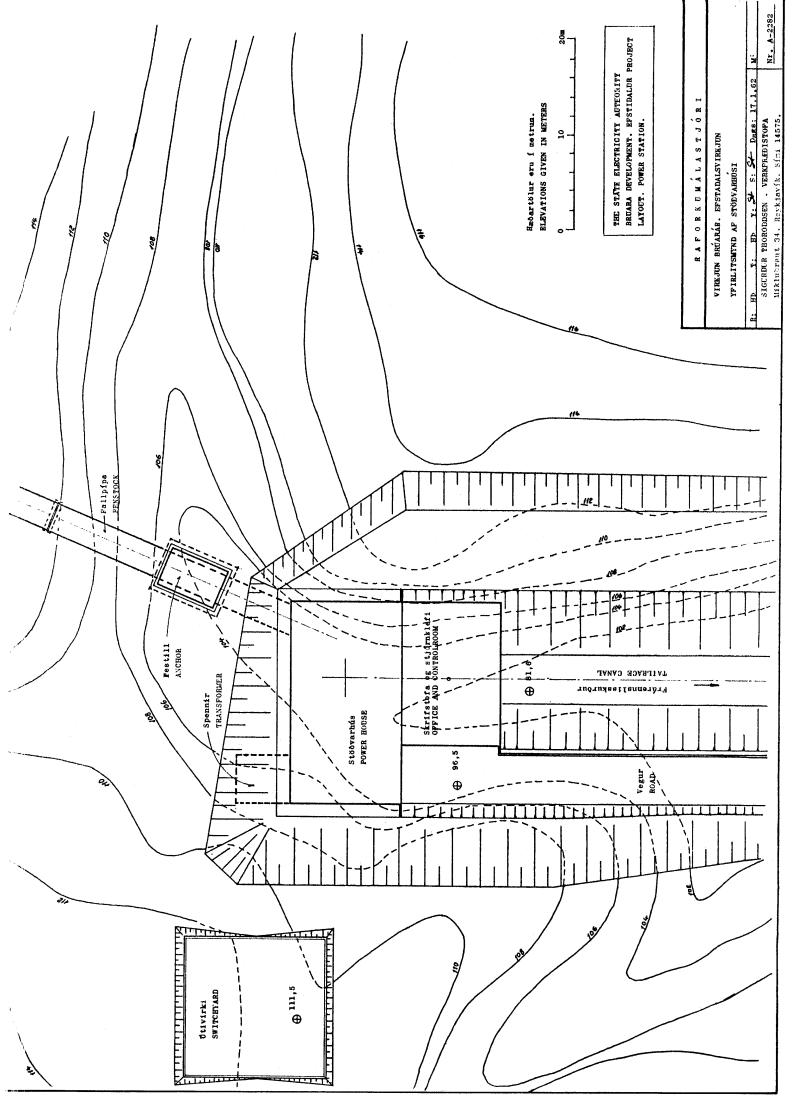
Turbine with accessories		
One Francis turbine with vertical shaft 25.800 H.P. (m), 250 RPM., together with necessary accessories	Mcr.	11.4
Machine hall crane and misc. mechanical		
equipment	Mer.	3.0
Generator with accessories		
One generator with vertical shaft 22.800 kVA; PF 0,8; 10,5 kV; 250 RPM, together with accessories	Mkr.	16.4
Accessory electrical equipment		3.5
Installation cost		-
	FIXT.	2.0
Total direct cost	EKT.	26.3
Grand Total, direct cost of production plant	Zkr.	.156.7
Transformer, 22.700/2000/22700 kVA, 10,5/11/55 kV Circuit breakers, meters, controlling equipment, etc.	Mer.	3.9 1.0
Installation	**	0.5
	MKP.	5.4
lie-in at Trafoss		
Transformer, 22.800 kVA; 66/38 kV Circuit breakers, meters, controlling	Max.	5.1
equipment, etc. Installation	Mcr.	0.5
	Mkr.	6.3
cansmission line		
frafons-Ffstidalur 29,2 km at kr. 300.000	rar.	8.8
Total cost, transmission plant	es en mas an m DINE.	20.5
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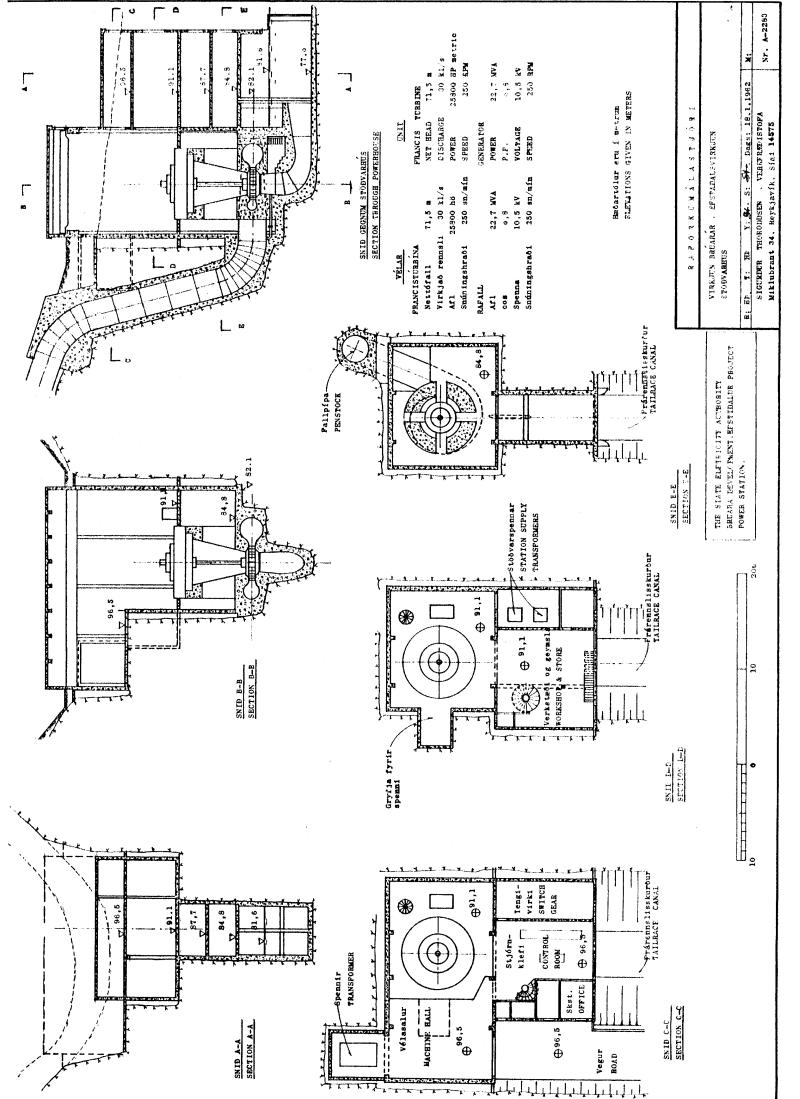


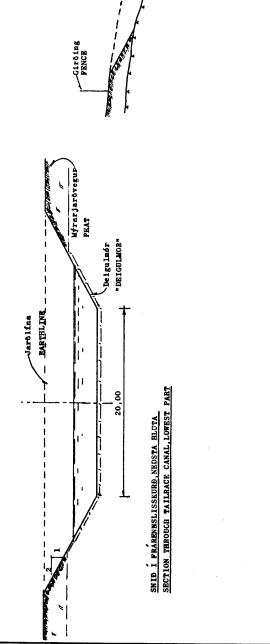








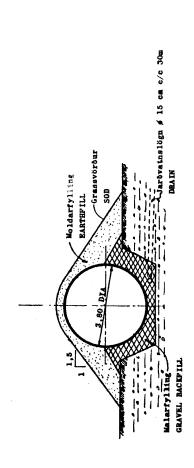




- Amtlue klapparhæe

ASSUMED ROCK

Tarblina EARTHLINE



SECTION THROUGH WOOD STAVE PIPE

SKID Í TREPÍPU

SNID I FRARENNSLISSKURD, EFRI HLUTA SECTION THROUGH TAILBACE CANAL, UPPER PART

5,00

THE STATE ELECTRICITY AUTEORITY
BRUARA DEVELOPMENT. EPSTIDALUR PROJECT
WOOD STAVE PIPE, TAILRACE CANAL.

RAFORKUMÁLASTJÓRI	VIRKJUN BRÚARÁR . BFSTADALSVIRKJUN	TREPIPA OG FRARENNSLISSKURDUR	P. Hb T. Hb V. C. S W. Dage. 18 1 1969
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MAI eru f metrum Dimensions given in Maters

Signatur 1: E. St. St. Peage: 18.1.1962 Signatur Thorodosen Verys redingrat Mikludizant 34. Reykjav (k. St. 14575.
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